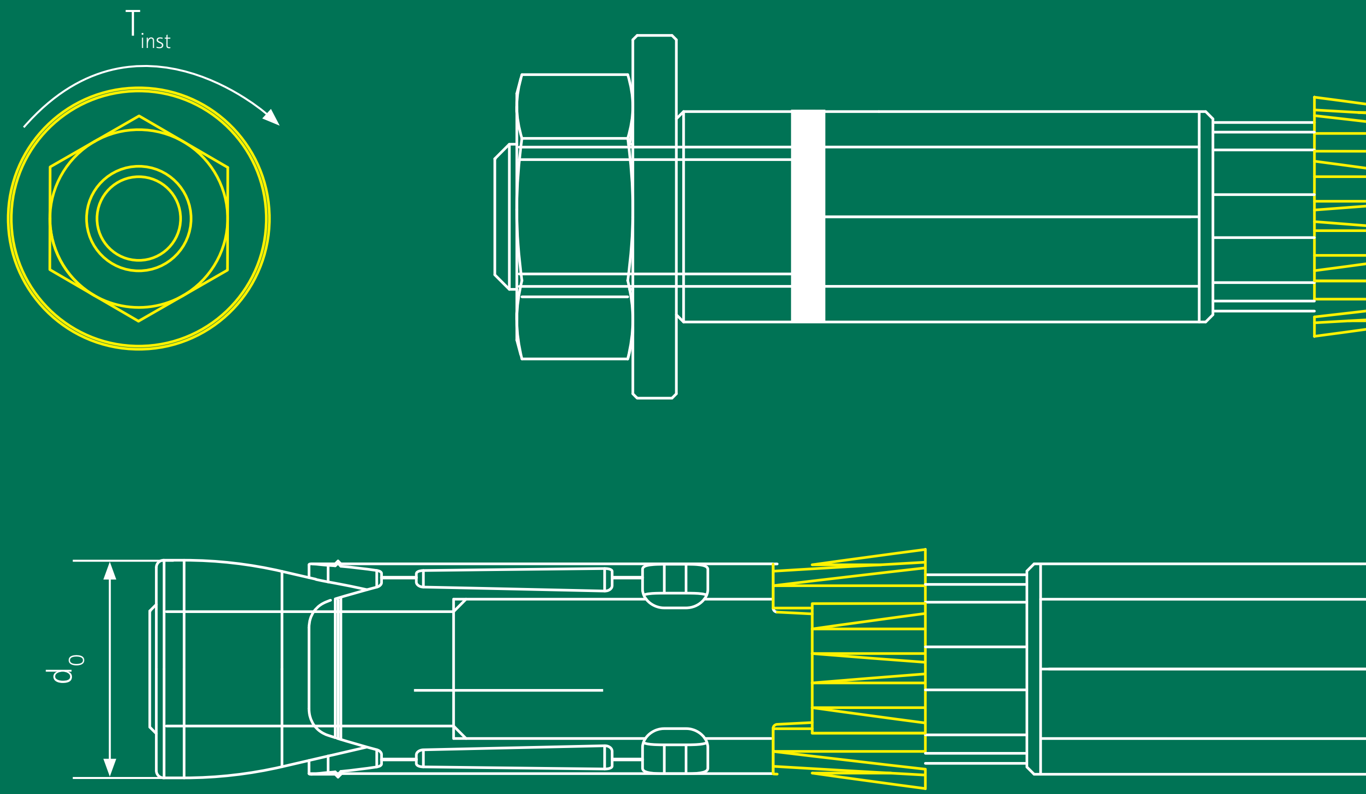
walraven

Approval



WHA1 Highload Anchor

ETA-16/0562

Deutsches Institut für Bautechnik
DIBt

Member of EOTA
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Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments

Designated according to Article 29 of Regulation (EU) No 305/2011 and member of EOTA (European Organisation for Technical Assessment)

European Technical Assessment

ETA-16/0562 of 15 July 2016

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:	Deutsches Institut für Bautechnik
Trade name of the construction product	Walraven Highload Anchor WHA1
Product family to which the construction product belongs	Torque controlled expansion anchor for use in concrete
Manufacturer	J. van Walraven Holding B.V. Industrieweg 5 3641 RK Mijdrecht NIEDERLANDE
Manufacturing plant	Walraven factory A5
This European Technical Assessment contains	20 pages including 3 annexes which form an integral part of this assessment
This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of	Guideline for European technical approval of "Metal anchors for use in concrete", ETAG 001 Part 2: "Torque controlled expansion anchors", April 2013, used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.

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Specific Part

1 Technical description of the product

The Walraven Highload Anchor WHA1 is an anchor made of galvanised steel or made of stainless steel which is placed into a drilled hole and anchored by torque-controlled expansion. The following anchor types are covered:

- Anchor type WHA1T with threaded bolt,
- Anchor type WHA1H with hexagon head screw,
- Anchor type WHA1C with countersunk washer and countersunk screw.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	**Performance**
Characteristic resistance for static and quasi-static loading	See Annex C1 to C5
Characteristic resistance for seismic performance category C1 and C2	See Annex C6 to C7
Displacements under tension and shear loads	See Annex C9 and C10

3.2 Safety in case of fire (BWR 2)

Essential characteristic	**Performance**
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	See Annex C8

3.3 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with guideline for European technical approval ETAG 001, April 2013 used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011 the applicable European legal act is: [96/582/EC].

The system to be applied is: 1

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

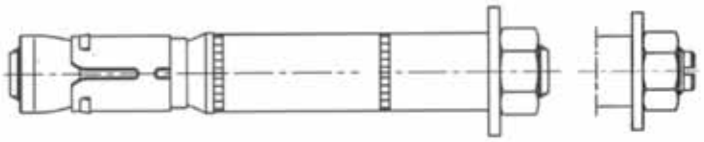
Issued in Berlin on 15 July 2016 by Deutsches Institut für Bautechnik

Andreas Kummerow
p.p. Head of Department

beglaubigt:
Lange

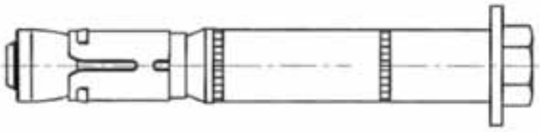
Highload Anchor WHA1

Anchor type WHA1T with threaded bolt



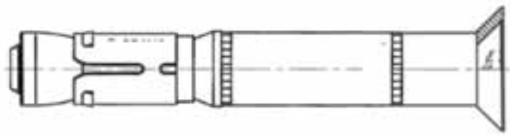
WHA1T (M6-M20)
WHA1T (M8-M16) A4

Anchor type WHA1H with hexagon head screw



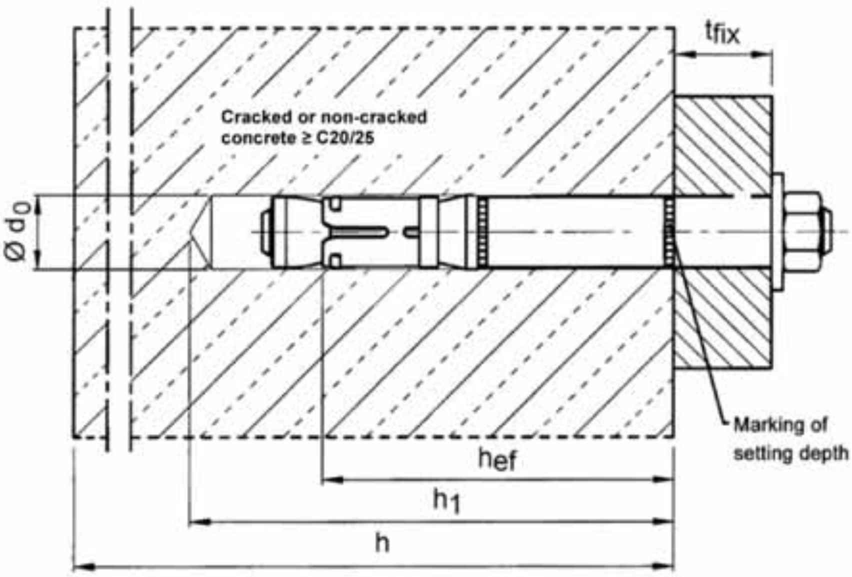
WHA1H (M6-M20)
WHA1H (M8-M16) A4

Anchor type WHA1C with countersunk washer and countersunk screw



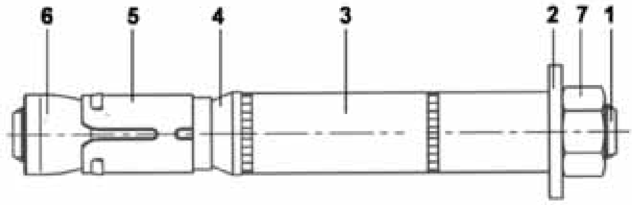
WHA1C (M6-M12)
WHA1C (M8-M12) A4

Installation condition

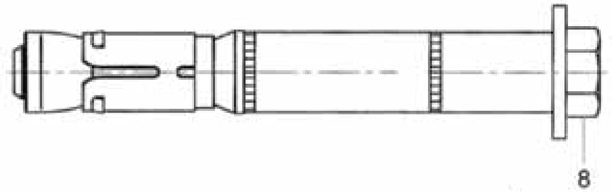


Walraven Highload Anchor WHA1	Annex A1
Product description Product and installation situation	

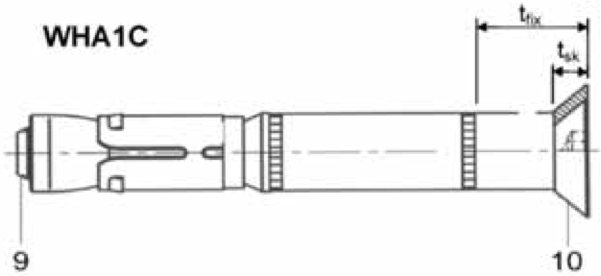
WHA1T



WHA1H



WHA1C



Marking:
expansion sleeve:

- Identifying mark of manufacturing plant	◇
- additional marking of stainless steel A4	A4
- Trade name (alternatively on distance sleeve)	SZ
- size of thread (alternatively on distance sleeve)	M10

Distance sleeve:

- Diameter	15
- max. thickness of fixture	25
- additional marking for countersunk version	SK

additional marking on the washer of anchor size WHA1 24/M16L	L

Table A1: Designation of anchor parts and materials

Part	Designation	Materials galvanised ≥ 5 µm, acc. to EN ISO 4042:1999	Stainless steel A4
1	Threaded bolt	Steel, Strength class 8.8, EN ISO 898-1:2013	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005
2	Washer	Steel, EN 10139:1997	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005
3	Distance sleeve	Precision steel tubes DIN 2394/2393	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005
4	Ring	Polyethylene	Polyethylene
5	Expansion sleeve	Steel, EN 10139:1997	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005
6	Threaded cone	Steel, Strength class 8, EN ISO 898-2:2012	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005
7	Hexagon nut	Steel, Strength class 8, EN ISO 898-2:2012	ISO 3506, strength class 70, stainless steel 1.4401 or 1.4571, EN 10088:2005
8	Hexagon head screw	Steel, Strength class 8.8, EN ISO 898-1:2013	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005
9	Countersunk screw	Steel, Strength class 8.8, EN ISO 898-1:2013	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005
10	Countersunk washer	Steel, EN 10083-2:2006	Stainless steel, 1.4401, 1.4404 or 1.4571, EN 10088:2005

Walraven Highload Anchor WHA1

Product description
Marking and materials

Annex A2

Specifications of intended use

Highload Anchor WHA1, steel zinc plated	10/M6	12/M8	15/M10	18/M12	24/M16	24/M16L	28/M20
Static or quasi-static action	✓						
Seismic action (WHA1T and WHA1H)	-	C1 + C2					
Fire exposure	R 30 … R 120						

Highload Anchor WHA1, stainless steel A4	12/M8	15/M10	18/M12	24/M16
Static or quasi-static action	✓			
Seismic action (WHA1T and WHA1H)	C1 + C2			
Fire exposure	R30 … R120			

Base materials:

- Cracked and non-cracked concrete
- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000
- Strength classes C20/25 to C50/60 according to EN 206-1:2000

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc plated steel or stainless steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel).

  Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used.)

Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages under static or quasi-static actions are designed in accordance with:
  - ETAG 001, Annex C, design method A, Edition August 2010 or
  - CEN/TS 1992-4:2009, design method A
- Anchorages under seismic actions (cracked concrete) are designed in accordance with:
  - EOTA Technical Report TR 045, Edition February 2013
  - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure
  - Fastenings in stand-off installation or with a grout layer are not allowed
- Anchorages under fire exposure are designed in accordance with:
  - EOTA Technical Report TR 020, Edition May 2004 or
  - CEN/TS 1992-4: 2009, Annex D

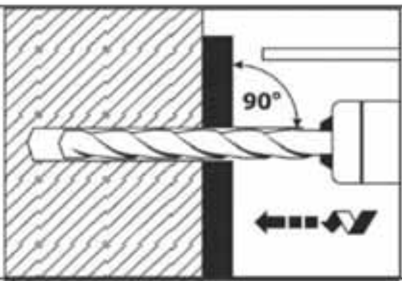
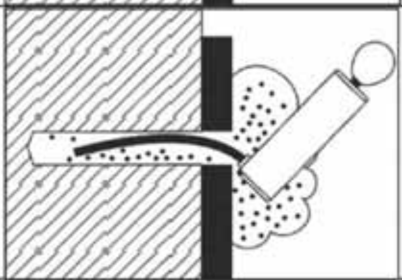
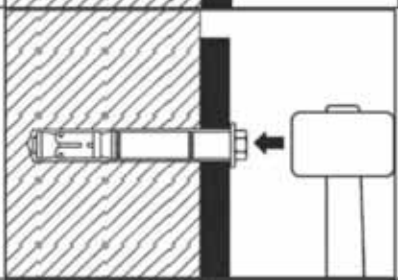
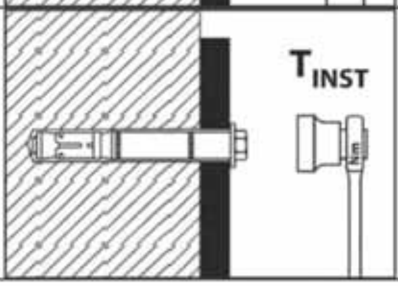
  (It must be ensured that local spalling of the concrete cover does not occur)

Installation:

- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.
- In case of aborted hole: new drilling at a minimum distance away of twice the depth of the aborted hole or smaller distance if the aborted drill hole is filled with high strength mortar and if under shear or oblique tension load it is not in the direction of load application.
- Anchor installation such that the effective anchorage depth is complied with. This compliance is ensured when the embedment mark of the anchor does no more exceed the concrete surface.

Walraven Highload Anchor WHA1	Annex B1
Intended use Specifications	

Installation instructions

1		Drill hole perpendicular to concrete surface.
2		Blow out dust.
3		Drive in anchor.
4		Apply tightening torque T_{inst} by using calibrated torque wrench.

Walraven Highload Anchor WHA1

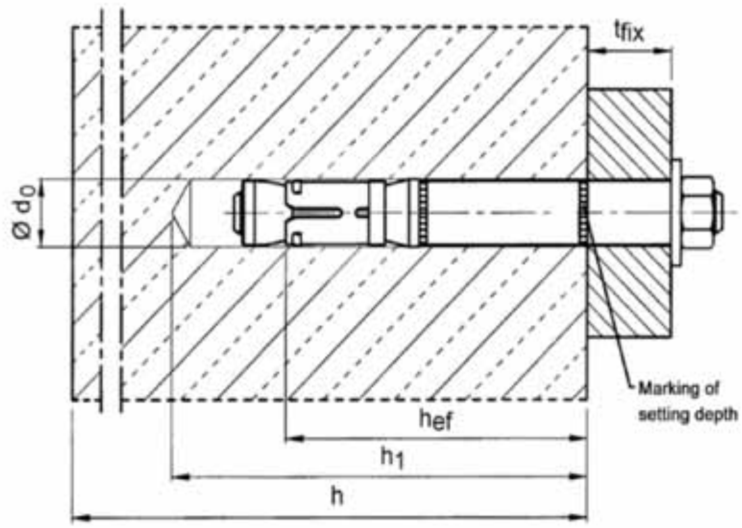
Intended use
Installation instructions

Annex B2

Table B1: Installation parameters, steel zinc plated

Anchor size			10/M6	12/M8	15/M10	18/M12	24/M16	24/M16L	28/M20
Size of thread		[-]	M6	M8	M10	M12	M16	M16	M20
Effective anchorage depth	h_{ef}	[mm]	50	60	71	80	100	115	125
Nominal diameter of drill bit	$d_0 =$	[mm]	10	12	15	18	24	24	28
Cutting diameter of drill bit	$d_{cut} \leq$	[mm]	10,45	12,5	15,5	18,5	24,55	24,55	28,55
Depth of drill hole	$h_1 \geq$	[mm]	65	80	95	105	130	145	160
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	12	14	17	20	26	26	31
Thickness of fixture WHA1T and WHA1H	$t_{fix\ min}$	[mm]	0	0	0	0	0	0	0
	$t_{fix\ max}$	[mm]	200	200	200	250	300	300	300
Thickness of fixture WHA1C	$t_{fix\ min}$ 2)	[mm]	8	10	14	18	-	-	-
	$t_{fix\ max}$	[mm]	200	200	200	250	-	-	-
Thickness of countersunk washer WHA1C	t_{sk}	[mm]	4	5	6	7	-	-	-
Required setting torque	T_{inst} (WHA1T, WHA1H)	[Nm]	15	30	50	80	160	160	280
	T_{inst} (WHA1C)	[Nm]	10	25	55	70	-	-	-
Minimum thickness of member	h_{min}	[mm]	100	120	140	160	200	230	250
Minimum spacing 1) 3)	s_{min}	[mm]	50	60	70	80	100	100	125
	for $c \geq$	[mm]	80	100	120	160	180	180	300
Minimum edge distance 1) 3)	c_{min}	[mm]	50	60	70	80	100	100	180
	for $s \geq$	[mm]	100	120	175	200	220	220	540

1) Intermediate values by linear interpolation
2) Depending on the existing shear load, the thickness of the fixture may be reduced to the thickness of the countersunk washer t_{sk} (see Annex A2). It must be verified that the present shear load can be transferred completely into the distance sleeve (bearing of hole).
3) For fire exposure from more than one side $c \geq 300$ mm or $c_{min} \geq 300$ mm applies.



Walraven Highload Anchor WHA1

Intended use
Installation parameters, **steel zinc plated**

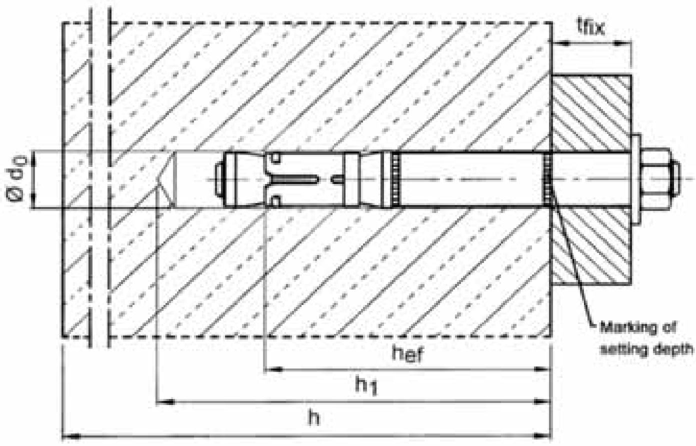
Annex B3

Deutsches Institut für Bautechnik DIBt

Table B2: Installation parameters, stainless steel A4

Anchor size			12/M8	15/M10	18/M12	24/M16
Size of thread		[-]	M8	M10	M12	M16
Effective anchorage depth	h_{ef}	[mm]	60	71	80	100
Nominal diameter of drill bit	$d_0 =$	[mm]	12	15	18	24
Cutting diameter of drill bit	$d_{cut} \leq$	[mm]	12,5	15,5	18,5	24,55
Depth of drill hole	$h_1 \geq$	[mm]	80	95	105	130
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	14	17	20	26
Thickness of fixture	$t_{fix\ min}$	[mm]	0	0	0	0
WHA1T and WHA1H	$t_{fix\ max}$	[mm]	200	200	250	300
Thickness of fixture	$t_{fix\ min}$ 2)	[mm]	10	14	18	-
WHA1C	$t_{fix\ max}$	[mm]	200	200	250	-
Thickness of countersunk washer WHA1C	t_{sk}	[mm]	5	6	7	-
Required setting torque	T_{inst} (WHA1T)	[Nm]	35	55	90	170
	T_{inst} (WHA1H)	[Nm]	30	50	80	170
	T_{inst} (WHA1C)	[Nm]	17,5	42,5	50	-
Minimum thickness of member	h_{min}	[mm]	120	140	160	200
Minimum spacing 1) 3)	s_{min}	[mm]	50	60	70	80
cracked concrete	for $c \geq$	[mm]	80	120	140	180
Minimum edge distance 1) 3)	c_{min}	[mm]	50	60	70	80
cracked concrete	for $s \geq$	[mm]	80	120	160	200
Minimum spacing 1) 3)	s_{min}	[mm]	50	60	70	80
non-cracked concrete	for $c \geq$	[mm]	80	120	140	180
Minimum edge distance 1) 3)	c_{min}	[mm]	50	85	70	180
non-cracked concrete	for $s \geq$	[mm]	80	185	160	80

1) Intermediate values by linear interpolation
2) Depending on the existing shear load, the thickness of the fixture may be reduced to the thickness of the countersunk washer t_{sk} (see Annex A2). It must be verified that the present shear load can be transferred completely into the distance sleeve (bearing of hole).
3) For fire exposure from more than one side $c \geq 300$ mm or $c_{min} \geq 300$ mm applies.



Walraven Highload Anchor WHA1

Intended use
Installation parameters, **stainless steel A4**

Annex B4

Table C1: Characteristic values for **tension load, cracked concrete** under static or quasi-static action, **steel zinc plated**

Anchor size			**10/M6**	**12/M8**	**15/M10**	**18/M12**	**24/M16**	**24/M16L**	**28/M20**
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0						
Steel failure									
Characteristic resistance	$N_{Rk,s}$	[kN]	16	29	46	67	126	126	196
Partial safety factor	γ_{Ms}	[-]	1,5						
Pull-out failure									
Characteristic resistance in cracked concrete C20/25	$N_{Rk,p}$	[kN]	5	12	16	1)	1)	1)	1)
Increasing factor for $N_{Rk,p}$	ψ_C	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$						
Concrete cone failure									
Effective anchorage depth	h_{ef}	[mm]	50	60	71	80	100	115	125
Factor acc. to CEN/TS 1992-4	k_{cr}	[-]	7,2						

1) Pull-out is not decisive.

Table C2: Characteristic values for **tension load, cracked concrete** under static or quasi-static action, **stainless steel A4**

Anchor size			**12/M8**	**15/M10**	**18/M12**	**24/M16**
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0			
Steel failure						
WHA1T						
Characteristic resistance	$N_{Rk,s}$	[kN]	26	41	60	110
Partial safety factor	γ_{Ms}	[-]	1,5			
WHA1H and WHA1C						
Characteristic resistance	$N_{Rk,s}$	[kN]	26	41	60	110
Partial safety factor	γ_{Ms}	[-]	1,87			
Pull-out failure						
Characteristic resistance in cracked concrete C20/25	$N_{Rk,p}$	[kN]	9	16	1)	1)
Increasing factor for $N_{Rk,p}$	ψ_C	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$			
Concrete cone failure						
Effective anchorage depth	h_{ef}	[mm]	60	71	80	100
Factor acc. to CEN/TS 1992-4	k_{cr}	[-]	7,2			

1) Pull-out is not decisive.

Walraven Highload Anchor WHA1

Performance
Characteristic values for **tension load** in **cracked concrete** under static or quasi-static action

Annex C1

Table C3: Characteristic values for **tension load in non-cracked concrete**, under static or quasi-static action, **steel zinc plated**

Anchor size			**10/M6**	**12/M8**	**15/M10**	**18/M12**	**24/M16**	**24/M16L**	**28/M20**
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0						
Steel failure									
Characteristic resistance	$N_{Rk,s}$	[kN]	16	29	46	67	126	126	196
Partial safety factor	γ_{Ms}	[-]	1,5						
Pull-out failure									
Characteristic resistance in non-cracked concrete C20/25	$N_{Rk,p}$	[kN]	1)	20	30	1)	1)	1)	1)
Splitting failure (The higher resistance of Case 1 and Case 2 may be applied.)									
Case 1									
Characteristic resistance in concrete C20/25	$N^0_{Rk,sp}$	[kN]	12 2)	16 2)	25 2)	30 2)	40 2)	70	50 2)
Spacing	$s_{cr,sp}$	[mm]	3 h_{ef}						
Edge distance	$c_{cr,sp}$	[mm]	1,5 h_{ef}						
Case 2 (acc. to ETAG 001, Annex C, equation (5.3))									
Spacing	$s_{cr,sp}$	[mm]	5 h_{ef}					3 h_{ef}	5 h_{ef}
Edge distance	$c_{cr,sp}$	[mm]	2,5 h_{ef}					1,5 h_{ef}	2,5 h_{ef}
Increasing factor for $N_{Rk,p}$ and $N^0_{Rk,sp}$	ψ_c	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$						
Concrete cone failure									
Effective Anchorage depth	h_{ef}	[mm]	50	60	71	80	100	115	125
Factor acc. to CEN/TS 1992-4	k_{ucr}	[-]	10,1						

1) Pull-out is not decisive.

2) For the proof against splitting failure, $N^0_{Rk,c}$ has to be has to be replaced by $N^0_{Rk,sp}$.

Walraven Highload Anchor WHA1	
Performance Characteristic values for **tension load** in **non-cracked concrete,** under static or quasi-static action, **steel zinc plated**	**Annex C2**

Table C4: Characteristic values for **tension load** in **non-cracked concrete** under static or quasi-static action, **stainless steel A4**

Anchor size			**12/M8**	**15/M10**	**18/M12**	**24/M16**
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0			
Steel failure						
WHA1T						
Characteristic resistance	$N_{Rk,s}$	[kN]	26	41	60	110
Partial safety factor	γ_{Ms}	[-]	1,5			
WHA1H and WHA1C						
Characteristic resistance	$N_{Rk,s}$	[kN]	26	41	60	110
Partial safety factor	γ_{Ms}	[-]	1,87			
Pull-out failure						
Characteristic resistance in non-cracked concrete C20/25	$N_{Rk,p}$	[kN]	16	25	35	[1]
Increasing factor for $N_{Rk,p}$	ψ_C	[-]	$\left(\frac{f_{ck,cube}}{25}\right)^{0,5}$			
Splitting failure						
Spacing	$s_{cr,sp}$	[mm]	360	470	530	600
Edge distance	$c_{cr,sp}$	[mm]	180	235	265	300
Concrete cone failure						
Effective anchorage depth	h_{ef}	[mm]	60	71	80	100
Factor acc. to CEN/TS 1992-4	k_{ucr}	[-]	10,1			

[1] Pull-out is not decisive.

Walraven Highload Anchor WHA1

Performance
Characteristic values for **tension loads** in **non-cracked concrete** under static or quasi-static action, **stainless steel A4**

Annex C3

Deutsches Institut für Bautechnik DIBt

Table C5: Characteristic values of **shear load** under static or quasi-static action, **steel zinc plated**

Anchor size			**10/M6**	**12/M8**	**15/M10**	**18/M12**	**24/M16**	**24/M16L**	**28/M20**
Steel failure without lever arm									
WHA1T									
Characteristic resistance	$V_{Rk,s}$	[kN]	16	25	36	63	91	91	122
Ductility factor	k_2	[-]	1,0						
Partial safety factor	γ_{Ms}	[-]	1,25						
WHA1H and WHA1C									
Characteristic resistance	$V_{Rk,s}$	[kN]	18	30	48	73	126	126	150
Ductility factor	k_2	[-]	0,8						
Partial safety factor	γ_{Ms}	[-]	1,25						
Steel failure with lever arm									
Characteristic resistance	$M^0_{Rk,s}$	[Nm]	12	30	60	105	266	266	519
Partial safety factor	γ_{Ms}	[-]	1,25						
Concrete pry-out failure									
Factor k acc. to ETAG 001, Annex C or k_3 acc. to CEN/TS 1992-4	$k_{(3)}$	[-]	1,8	2,0					
Concrete edge failure									
Effective length of anchor in shear loading	l_f	[mm]	50	60	71	80	100	115	125
Outside diameter of anchor	d_{nom}	[mm]	10	12	15	18	24	24	28

Walraven Highload Anchor WHA1

Performance
Characteristic values for **shear load** under static or quasi-static action,
steel zinc plated

Annex C4

Table C6: Characteristic values for **shear load** under static or quasi-static action, **stainless steel A4**

Anchor size			12/M8	15/M10	18/M12	24/M16
Steel failure without lever arm						
WHA1T						
Characteristic resistance	$V_{Rk,s}$	[kN]	24	37	62	92
Ductility factor	k_2	[-]	1,0			
Partial safety factor	γ_{Ms}	[-]	1,25			
WHA1H and WHA1C						
Characteristic resistance	$V_{Rk,s}$	[kN]	24	37	62	92
Ductility factor	k_2	[-]	0,8			
Partial safety factor	γ_{Ms}	[-]	1,36			
Steel failure with lever arm						
WHA1T						
Characteristic resistance	$M^0_{Rk,s}$	[Nm]	26	52	92	232
Ductility factor	k_2	[-]	1,0			
Partial safety factor	γ_{Ms}	[-]	1,25			
WHA1H and WHA1C						
Characteristic resistance	$M^0_{Rk,s}$	[Nm]	26	52	92	232
Ductility factor	k_2	[-]	0,8			
Partial safety factor	γ_{Ms}	[-]	1,56			
Concrete pry-out failure						
Factor k acc. to ETAG 001, Annex C or k_3 acc. to CEN/TS 1992-4	$k_{(3)}$	[-]	2,0			
Concrete edge failure						
Effective length of anchor in shear loading	l_f	[mm]	60	71	80	100
Outside diameter of anchor	d_{nom}	[mm]	12	15	18	24

Walraven Highload Anchor WHA1

Performance
Characteristic values for **shear load** under static or quasi-static action, **stainless steel A4**

Annex C5

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Table C7: Characteristic values for **seismic action, Category C1 and C2, steel zinc plated**

Anchor size			12/M8	15/M10	18/M12	24/M16	24/M16L	28/M20
Tension load								
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0					
Steel failure								
Characteristic tension resistance category **C1**	$N_{Rk,s,seis,C1}$	[kN]	29	46	67	126	126	196
Characteristic tension resistance category **C2**	$N_{Rk,s,seis,C2}$	[kN]	29	46	67	126	126	196
Partial safety factor	$\gamma_{Ms,seis}$	[-]	1,5					
Pull-out failure								
Characteristic tension resistance category **C1**	$N_{Rk,p,seis,C1}$	[kN]	12	16	25	36	44,4	50,3
Characteristic tension resistance category **C2**	$N_{Rk,p,seis,C2}$	[kN]	5,4	16,4	22,6	29,0	41,2	43,6
Increasing factor for $N_{Rk,p,seis}$	ψ_c	[-]	1,0					
Shear load								
Steel failure without lever arm								
WHA1T								
Characteristic shear resistance category **C1**	$V_{Rk,s,seis,C1}$	[kN]	18,0	27,1	43,4	51,9	51,9	96,4
Characteristic shear resistance category **C2**	$V_{Rk,s,seis,C2}$	[kN]	12,7	20,5	31,5	50,1	50,1	67,1
WHA1H								
Characteristic shear resistance category **C1**	$V_{Rk,s,seis,C1}$	[kN]	18,0	27,1	43,4	51,9	51,9	96,4
Characteristic shear resistance category **C2**	$V_{Rk,s,seis,C2}$	[kN]	12,7	20,5	31,5	69,3	69,3	67,1
Partial safety factor	$\gamma_{Ms,seis}$	[-]	1,25					
Steel failure with lever arm								
Characteristic resistance	$M^0_{Rk,s,seis}$	[Nm]	no performance determined					

Walraven Highload Anchor WHA1

Performance
Characteristic values for **seismic action, steel zinc plated**

Annex C6

Table C8: Characteristic values for **seismic action, Category C1 and C2, stainless steel A4**

Anchor size			**12/M8**	**15/M10**	**18/M12**	**24/M16**
Tension load						
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0			
Steel failure						
Characteristic tension resistance, category **C1**	$N_{Rk,s,seis,C1}$	[kN]	26	41	60	110
Characteristic tension resistance, category **C2**	$N_{Rk,s,seis,C2}$	[kN]	26	41	60	110
Partial safety factor **WHA1T**	$\gamma_{Ms,seis}$	[-]	1,5			
Partial safety factor **WHA1H**	$\gamma_{Ms,seis}$	[-]	1,87			
Pull-out failure						
Characteristic tension resistance, category **C1**	$N_{Rk,p,seis,C1}$	[kN]	9	16	26	36
Characteristic tension resistance, category **C2**	$N_{Rk,p,seis,C2}$	[kN]	4,8	16,5	24,8	44,5
Increasing factor for $N_{Rk,p,seis}$	ψ_c	[-]	1,0			
Shear load						
Steel failure without lever arm						
Characteristic shear resistance, category **C1**	$V_{Rk,s,seis,C1}$	[kN]	9,6	13,3	25,4	75,4
Characteristic shear resistance, category **C2**	$V_{Rk,s,seis,C2}$	[kN]	9,7	14,0	18,0	32,2
Partial safety factor **WHA1T**	$\gamma_{Ms,seis}$	[-]	1,25			
Partial safety factor **WHA1H**	$\gamma_{Ms,seis}$	[-]	1,36			
Steel failure with lever arm						
Characteristic resistance	$M^0_{Rk,s,seis}$	[Nm]	no performance determined			

Walraven Highload Anchor WHA1

Performance
Characteristic values for **seismic action, stainless steel A4**

Annex C7

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Table C9: Characteristic values for **tension and shear load** under **fire exposure** in cracked and non-cracked concrete C20/25 to C50/60

Anchor size				**10/M6**	**12/M8**	**15/M10**	**18/M12**	**24/M16**	**24/M16L**	**28/M20**
Tension load										
Steel failure										
Steel zinc plated										
Characteristic resistance	R30	$N_{Rk,s,fi}$	[kN]	1,0	1,9	4,3	6,3	11,6		18,3
	R60			0,8	1,5	3,2	4,6	8,6		13,5
	R90			0,6	1,0	2,1	3,0	5,0		7,7
	R120			0,4	0,8	1,5	2,0	3,1		4,9
Stainless steel A4										
Characteristic resistance	R30	$N_{Rk,s,fi}$	[kN]	-	6,1	10,2	15,7	29,2	-	-
	R60			-	4,4	7,3	11,1	20,6	-	-
	R90			-	2,6	4,3	6,4	12,0	-	-
	R120			-	1,8	2,8	4,1	7,7	-	-
Shear load										
Steel failure without lever arm										
Steel zinc plated										
Characteristic resistance	R30	$V_{Rk,s,fi}$	[kN]	1,0	1,9	4,3	6,3	11,6		18,3
	R60			0,8	1,5	3,2	4,6	8,6		13,5
	R90			0,6	1,0	2,1	3,0	5,0		7,7
	R120			0,4	0,8	1,5	2,0	3,1		4,9
Stainless steel A4										
Characteristic resistance	R30	$V_{Rk,s,fi}$	[kN]	-	14,3	22,7	32,8	61,0	-	-
	R60			-	11,1	17,6	25,5	47,5	-	-
	R90			-	7,9	12,6	18,3	34,0	-	-
	R120			-	6,3	10,0	14,6	27,2	-	-
Steel failure with lever arm										
Steel zinc plated										
Characteristic resistance	R30	$M^0_{Rk,s,fi}$	[Nm]	0,8	2,0	5,6	9,7	24,8		42,4
	R60			0,6	1,5	4,1	7,2	18,3		29,8
	R90			0,4	1,0	2,7	4,7	11,9		17,1
	R120			0,3	0,8	1,9	3,1	6,6		10,7
Stainless steel A4										
Characteristic resistance	R30	$M^0_{Rk,s,fi}$	[Nm]	-	6,2	13,2	24,4	61,8	-	-
	R60			-	4,5	9,4	17,2	43,6	-	-
	R90			-	2,7	5,6	10,0	25,3	-	-
	R120			-	1,8	3,6	6,4	16,2	-	-

The characteristic resistances for pull-out failure, concrete cone failure, concrete pry-out and concrete edge failure can be calculated according to TR020 / CEN/TS 1992-4.

Walraven Highload Anchor WHA1

Performance
Characteristic values for **tension and shear loads** under **fire exposure**

Annex C8

Table C10: Displacements under **tension load**

Anchor size			**10/M6**	**12/M8**	**15/M10**	**18/M12**	**24/M16**	**24/M16L**	**28/M20**
Steel, zinc plated									
Tension load in cracked concrete	N	[kN]	2,4	5,7	7,6	12,3	17,1	21,1	24
Displacement	δ_{N0}	[mm]	0,5	0,5	0,5	0,7	0,8	0,7	0,9
	$\delta_{N\infty}$	[mm]	2,0	2,0	1,3	1,3	1,3	1,3	1,4
Tension load in non-cracked concrete	N	[kN]	8,5	9,5	14,3	17,2	24	29,6	34
Displacement	δ_{N0}	[mm]	0,8	1,0	1,1			1,3	0,3
	$\delta_{N\infty}$	[mm]	3,4		1,7			2,3	1,4
Seismic action C2									
Displacement for DLS	$\delta_{N,seis,C2(DLS)}$	[mm]	-	3,3	3,0	5,0	3,0	3,0	4,0
Displacement for ULS	$\delta_{N,seis,C2(ULS)}$	[mm]	-	12,2	11,3	16,0	9,2	9,2	13,8
Stainless steel A4									
Tension load in cracked concrete	N	[kN]	-	4,3	7,6	12,1	17,0	-	-
Displacement	δ_{N0}	[mm]	-	0,5	0,5	1,3	0,5	-	-
	$\delta_{N\infty}$	[mm]	-	1,2	1,6	1,8	1,6	-	-
Tension load in non-cracked concrete	N	[kN]	-	7,6	11,9	16,7	24,1	-	-
Displacement	δ_{N0}	[mm]	-	0,2	0,3	1,2	1,5	-	-
	$\delta_{N\infty}$	[mm]	-	1,1				-	-
Seismic action C2									
Displacement for DLS	$\delta_{N,seis,C2(DLS)}$	[mm]	-	4,7	4,5	4,3	4,9	-	-
Displacement for ULS	$\delta_{N,seis,C2(ULS)}$	[mm]	-	13,3	12,7	9,7	10,1	-	-

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Performance
Displacements under **tension load**

Annex C9

Table C11: Displacements under **shear load**

Anchor size			10/M6	12/M8	15/M10	18/M12	24/M16	24/M16L	28/M20
Steel, zinc plated									
WHA1T									
Shear load in cracked and non-cracked concrete	V	[kN]	9,1	14	20,7	35,1	52,1	52,1	77
Displacement	δ_{V0}	[mm]	2,5	2,1	2,7	3,0	5,1	5,1	4,3
	$\delta_{V\infty}$	[mm]	3,8	3,1	4,1	4,5	7,6	7,6	6,5
Seismic action C2									
Displacement for DLS	$\delta_{V,seis,C2(DLS)}$	[mm]	-	2,3	3,1	3,0	2,6	2,6	1,6
Displacement for ULS	$\delta_{V,seis,C2(ULS)}$	[mm]	-	4,8	6,4	6,1	6,6	6,6	4,8
WHA1H and WHA1C									
Shear load in cracked and non-cracked concrete	V	[kN]	10,1	17,1	27,5	41,5	72	72	77
Displacement	δ_{V0}	[mm]	2,9	2,5	3,6	3,5	7,0	7,0	4,3
	$\delta_{V\infty}$	[mm]	4,4	3,8	5,4	5,3	10,5	10,5	6,5
Seismic action C2 (WHA1H)									
Displacement for DLS	$\delta_{V,seis,C2(DLS)}$	[mm]	-	2,3	3,1	3,0	3,3	3,3	1,6
Displacement for ULS	$\delta_{V,seis,C2(ULS)}$	[mm]	-	4,8	6,4	6,1	8,2	8,2	4,8
Stainless steel A4									
Shear load in cracked and non-cracked concrete	V	[kN]	-	13,9	21,1	34,7	50,8	-	-
Displacement	δ_{V0}	[mm]	-	3,4	4,9	4,8	6,7	-	-
	$\delta_{V\infty}$	[mm]	-	5,1	7,4	7,1	10,1	-	-
Seismic action C2									
Displacement for DLS	$\delta_{V,seis,C2(DLS)}$	[mm]	-	2,8	3,1	2,6	3,3	-	-
Displacement for ULS	$\delta_{V,seis,C2(ULS)}$	[mm]	-	5,6	5,8	5,0	6,9	-	-

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Performance
Displacements under **shear load**

Annex C10

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